Planning Proposal Proposed Mixed Use Development

124-142 Beamish Street, Campsie

TRAFFIC AND PARKING ASSESSMENT REPORT

3 April 2020

Ref 18718



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1. INTRODUCTION

This report has been prepared to accompany a planning proposal for a mixed-use development to be located at 124-142 Beamish Street, Campsie (Figures 1 and 2)

The planning proposal seeks approval to amend the planning controls in order to permit a mixed-use development with an increased apartment yield. Off-street car parking will ultimately be provided in a new multi-level basement car parking area beneath the building in accordance with *SEPP 65* and Council requirements.

The site is located within the heart of the "Campsie Station Precinct" as outlined in the Department of Planning & Environment's *Sydenham to Bankstown Urban Renewal Corridor Strategy* document. The document details the redevelopment objectives of the corridor and comprises a range of uses, including low/medium/high residential, retail/commercial premises, schools and recreational facilities. The document also identifies improvements to alternate forms of travel including pedestrian and cycle paths as well as public transport.

The purpose of this report is to assess the traffic and parking implications of the planning proposal and to that end this report:

- describes the site and provides details of the planning proposal
- reviews the road network in the vicinity of the site, and the traffic conditions on that road network
- reviews the alternate forms of transport available in the vicinity of the site
- estimates the traffic generation potential of the planning proposal, and assigns that traffic generation to the road network serving the site
- assesses the traffic implications of the development proposal in terms of road network capacity

- reviews the geometric design features of the proposed car parking and loading facilities for compliance with the relevant codes and standards
- assesses the adequacy and suitability of the quantum of off-street car parking and loading envisaged on the site.





2. PLANNING PROPOSAL

Site

The subject site is located on the western side of Beamish Street, extending between Ninth Avenue and Campsie Street. The site occupies an area of approximately 3,845m² and has street frontages of approximately 75m in length to Ninth Avenue, 52m in length to Beamish Street and 60m in length to Campsie Street.

The subject site is currently occupied by a mix of two-storey retail and commercial buildings comprising a cumulative floor area of approximately 4,745m².

At-grade off-street parking is provided at the rear of the buildings, with vehicular access provided via driveways located off the Campsie Street, Beamish Street and Ninth Avenue site frontages.

A recent aerial image of the site and its surroundings is reproduced below.



Existing Planning Controls

The primary instrument that governs the mass and scale of the development on the site are contained within the *Canterbury Local Environment Plan 2012 (CLEP2012)*.

The subject site is zoned B2 - Local Centre and is not subject to any FSR control. The scale of any development on the site is currently constrained by a building height of 21m.

It is therefore envisaged that a mixed-use development comprising 240 apartments, $3,000m^2$ of retail floor space and $2,500m^2$ of commercial floor space is achievable under the existing planning controls of the site.

Sydenham to Bankstown Urban Renewal Corridor

As outlined in the Department of Planning & Environment's *Sydenham to Bankstown Urban Renewal Corridor Strategy* document, the NSW Government plans for 35,400 new homes and 8,700 new jobs over the next 20 years and infrastructure to support the future community's needs. The strategy builds on the Sydney Metro City and Southwest project and provides a co-ordinated approach to infrastructure delivery and development across the corridor, with the project's priorities summarised below:

- provide a range of new homes to suit different lifestyles and budgets
- allow for new homes to be built close to the metro stations
- retail streets that contribute to the local character of the area and protect heritage areas
- retail the scale and character of popular local shopping areas
- ensure adequate schools and childcare centres are provided for the future increased population
- identify areas for pedestrian and cycle paths including the potential for GreenWay South West
- provide a significant increase in transport capacity with new, faster and more frequent services

Campsie Station Precinct

The site is located within the heart of the "Campsie Station Precinct" as outlined in Department of Planning & Environment's *Sydenham to Bankstown Urban Renewal Corridor Strategy* document.

The document details the redevelopment objectives of the corridor and comprises a range of uses, including low/medium/high residential, retail/commercial premises, schools and recreational facilities. The document also identifies improvements to alternate forms of travel including pedestrian and cycle paths as well as public transport.





Sydenham to Bankstown Urban Renewal Corridor Strategy

The Sydney Metro works include improvement to the existing entry of the railway station along with public domain and access improvements, with increased footpath widths which will encourage local retail activity surrounding the railway station.

Along with the new Metro service, the strategy proposes to introduce bus priority measures along Beamish Street, Seventh Avenue and Fifth Avenue for the 409 and M41 services, while increased service levels are also proposed for the 487 service between Canterbury local centre and Bankstown CBD. The modal changes between rail and bus networks will also be improved with an upgraded interchange at Campsie railway station.

The completion of the western "Campsie Bypass" at Loch Street will improve regional vehicle and bus connections to major roads and employment destinations and pedestrian amenity along Beamish Street.

Alternate forms of transport including walking, cycling and public transport will be further encouraged by improving the quality of the railway station for passengers with better access, facilities and signage as well as improving pedestrian and cycle access within the precinct. A new east-west regional cycle link along the rail corridor is also planned in order to improve pedestrian and cycle access between the town centres and railway stations along the corridor.

New streets and/or pedestrian connections are also proposed through larger blocks as they develop, enhancing the permeability of the current vehicular and pedestrian network within the Campsie Station Precinct, including a new link between Ninth Avenue and Campsie Street.

Planning Proposal

The planning proposal seeks approval to amend the current planning controls which apply to the site in order to increase the site's development yield, permitting a mixed-use development comprising buildings ranging in height from 4 storeys to 25 storeys.

For the purposes of this traffic assessment it has been assumed that the proposed mixed-use building could comprise in the order of 320 apartments on the upper levels as follows:

161	2 bedroom apartments:
134	1 bedroom apartments:

In addition, approximately $3,145m^2$ of retail floor space and approximately $1,600m^2$ of commercial floor space is also proposed on the lower levels.

Off-street car parking will ultimately be provided in a new multi-level basement car parking area beneath the buildings, designed to comply with *SEPP 65* and Council's requirements as well as the relevant Australian Standards. Vehicular access to the site is proposed to be provided via a new entry/exit driveway to be located at the western end of the Ninth Avenue site frontage.

Loading/servicing for the proposed development is expected to be undertaken by a variety of commercial vehicles up to and including 8.8m long medium rigid trucks. A dedicated loading area is to be located on the ground level at the rear of the retail tenancies and fitted with a mechanical turntable, thereby allowing all trucks to enter and exit the site in a forward direction at all times. Vehicular access to the loading area is to be provided via a dedicated service driveway which is to be located adjacent to the abovementioned proposed basement site access driveway off Ninth Avenue.

Plans for the purposes of this planning proposal have been prepared by *Turner Architects* and are reproduced in the following pages.















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3. TRAFFIC ASSESSMENT

Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services is illustrated on Figure 3.

Canterbury Road is classified by the RMS as a *State Road* and provides the key east-west road link in the area, linking Bankstown and Hurlstone Park. It typically carries two traffic lanes in each direction in the vicinity of the site, with clearway restrictions applying along both sides of the road during commuter peak periods.

Beamish Street and Brighton Avenue is classified by the RMS as a *Regional Road* which provides the key north-south road link in the area, linking Canterbury Road to Georges River Road. The route typically carries one traffic lane in each direction in the vicinity of the site and kerbside parking is permitted at selected locations along the road.

Ninth Avenue is also classified by the RMS as a *Regional Road* which provides the key eastwest road link in the area, linking Campsie to Wiley Park via Albert Street and Lakemba Street. It typically carries one traffic lane in each direction in the vicinity of the site and kerbside parking is generally permitted on one or both sides of the road, subject to sign posted restrictions.

Campsie Street is a local, unclassified road which is primarily used to provide vehicular and pedestrian access to frontage properties. Kerbside parking is generally permitted along both sides the road, subject to sign posted restrictions.

Existing Traffic Controls

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

 a 40 km/h SPEED LIMIT which applies to Beamish Street and all other local roads in the Campsie City Centre





- TRAFFIC SIGNALS in Beamish Street where it intersects with Ninth Avenue, Clissold Parade and also South Parade/Lilian Street
- RIGHT TURN HOLDING BAYS in Beamish Street where it intersects with Ninth Avenue and also Campsie Street
- a NO RIGHT-TURN westbound restriction in Ninth Avenue onto Sixth Avenue
- a LEFT-TURN ONLY southbound restriction in Sixth Avenue onto Ninth Avenue
- RAISED PEDESTRIAN CROSSINGS in Campsie Street and also North Parade in the vicinity of Beamish Street.

Existing Public Transport Services

The existing public transport services available to the site are illustrated on Figure 5.

The subject site is conveniently located within 200 metres of walking distance to the entrance of Campsie Railway Station. The station lies on the T3 Bankstown Line which operates between Bankstown and Sydney CBD. Services generally operate at a frequency of approximately 5-10 minutes during commuter peak periods and 15-30 minutes at other times.

In addition to the train services, a number of bus routes currently operate along Beamish Street with bus stops located within a 100 metres walking distance from the subject site. Notably, the M41 bus route currently operates along Beamish Street which is part of the Sydney's *Metrobus* network that provides high-frequency, high-capacity links between key employment and growth centres across Sydney. The M41 links between Hurstville, Bexley North, Campsie, Burwood, Concord, Rhodes, Ryde, Top Ryde, North Ryde, Macquarie park and Macquarie Centre operating at 10-minute intervals during commuter peak periods, 15-minute intervals during the day and 20-minute intervals at other times.

Other bus services available in the vicinity of the subject site includes route 400, 412, 413, 415, 444, 445, 473, 487, 490, 492 and 942.



The site lies within the heart of the Campsie town centre which includes a wide range of essential shops and services such as a mini-market, fruit market, butchery, bakery, seafood shop, bottle shop, post office, pharmacy, optometrist, newsagency, hair dresser and beautician.

The site is therefore considered to be highly accessible to essential services and public transport options.

Local Bicycle Routes

The location of the existing and proposed bicycle routes in the vicinity of the site are illustrated on the figure below, with the subject site marked with a star. These bicycle routes are readily accessible from the subject site and provide a number of on-road and off-road bicycle routes linking the local area with the greater surrounding area. The proposed development will make provision for a substantial bicycle parking area which is to be located within the basement level and will enhance the *active* transport options available to future occupants of the site.



Sydney Metro City & Southwest

As described on the Transport for NSW website, the new Sydney Metro City & Southwest rail line is one of the NSW Government's largest infrastructure projects being delivered to serve a growing Sydney. Regular services will be provided from early morning to late evening, including every 4 minutes during peak periods, and there will not be a timetable, customers simply turn up and go.

Stage 1 "Norwest" will deliver a new 36km line, connecting Cudgegong Road to Chatswood and include 8 new metro stations, five upgraded stations and 4,000 commuter car parking spaces. Stage 1 is expected to open in the first half of 2019.

Stage 2 "City & Southwest" will deliver a 30km extension of the metro rail from Chatswood under Sydney Harbour, through new CBD stations and south-west to Bankstown. Stage 2 is due to open in 2024, with 7 new metro stations and 11 upgraded stations.



Sydney Metro alignment map

Existing Traffic Conditions

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken as part of this traffic study. The traffic surveys were during the morning and afternoon peak periods on Thursday 1st November, 2018 at the following intersections:

- Beamish Street and Ninth Avenue intersection
- Beamish Street and Campsie Street intersection.

The results of the traffic surveys are reproduced in full in Appendix A and reveal that:

- two-way traffic flows in Beamish Street are typically in the order of 1,300 vehicles per hour (vph) during peak periods
- two-way traffic flows in Ninth Avenue are typically in the order of 1,000 vph during peak periods
- two-way traffic flows in Campsie Street are typically in the order of 250 vph during peak periods.

Projected Traffic Generation

An indication of the traffic generation potential of the planning proposal is provided by reference to the Roads and Maritime Services publication *Guide to Traffic Generating Developments, Section 3 - Landuse Traffic Generation (October 2002)* and the updated traffic generation rates in the recently published RMS *Technical Direction (TDT 2013/04a)* document.

The *TDT 2013/04a* document specifies that it replaces those sections of the RMS *Guidelines* indicated, and that it must be followed when RMS is undertaken trip generation and/or parking demand assessments.

The RMS *Guidelines* and the updated *TDT 2013/04a* are based on extensive surveys of a wide range of land uses and nominate the following traffic generation rates which are applicable to the planning proposal:

Commercial Offices

- AM: 1.6 peak hour vehicle trips per 100m² GFA
- PM: 1.2 peak hour vehicle trips per $100m^2$ GFA

High Density Residential Flat Dwellings

- AM: 0.19 peak hour vehicle trips per unit
- PM: 0.15 peak hour vehicle trips per unit

The RMS *Guidelines* also make the following observation in respect of high density residential flat buildings:

Definition

A *high density residential flat building* refers to a building containing 20 or more dwellings. This does not include aged or disabled persons housing. *High density residential flat buildings* are usually more than 5 levels, have basement level car parking and are located in close proximity to public transport services. The building may contain a component of commercial use.

Factors

The above rates include visitors, staff, service/delivery and on-street movements such as taxis and pick-up/set-down activities.

However, the RMS *Guidelines* and the updated *TDT 2013/04a* do not nominate a traffic generation rate for small, local shops, referring only to major regional shopping centres incorporating supermarkets and department stores. For the purpose of this assessment therefore, the commercial traffic generation rate has been adopted in respect of the retail component of the planning proposal.

Application of the above traffic generation rates to the various components of the development proposal yields a traffic generation potential of approximately 137 vph during the weekday AM peak period and approximately 105 vph during the weekday PM peak period, as set out below:

	AM	PM
Residential (320 apartments):	61 vph	48 vph
Retail shops (3,145m ²):	50 vph	38 vph
Commercial offices (1,600m ²):	26 vph	19 vph
TOTAL TRAFFIC GENERATION POTENTIAL:	137 vph	105 vph

Planning Proposal - Projected Future Traffic Generation Potential

That projected future level of traffic generation potential which is expected to occur as a consequence of the planning proposal should however, be offset or *discounted* by the volume of traffic which could reasonably be expected to be generated by a development permitted by the current *CLEP2012* planning controls (in terms of height).

Application of the above traffic generation rates to the 240 residential apartments, 3,000m² of retail floor space and 2,500m² of commercial floor space achievable under the current *LEP 2012* planning controls yields a traffic generation potential of approximately 134 vph during the weekday AM peak period and approximately 102 vph during the weekday PM peak period, as set out below:

Current CLEP 2012 Planning Controls - Projected Future Traffic Generation Potential

	AM	PM
Residential (240 apartments):	46 vph	36 vph
Retail shops (3,000m ²):	48 vph	36 vph
Commercial offices (2,500m ²):	40 vph	30 vph
TOTAL TRAFFIC GENERATION POTENTIAL:	134 vph	102 vph

Accordingly, it is likely that the planning proposal will result in a *nett* increase in the traffic generation potential of the site of just 3 vph during the weekday AM and PM peak periods, when compared with the existing planning controls that apply to the site, as set out below:

Projected Nett Increase in Peak Hour Traffic Generation Potential
of the Site as a Consequence of the Planning Proposal

	AM	PM
Planning Proposal Traffic Generation Potential:	137 vph	105 vph
Existing Planning Controls Traffic Generation Potential:	-134 vph	-102 vph
NETT INCREASE IN TRAFFIC GENERATION POTENTIAL:	+3 vph	+3 vph

Notwithstanding, for the purposes of this assessment it has been assumed that *all* of the projected future traffic flows of 137 vph and 105 vph in the AM and PM commuter peak periods respectively, will be new or *additional* to the existing traffic flows currently using the adjacent road network.

That projected increase in the traffic generation potential of the site as a consequence of the planning proposal will not have any unacceptable traffic implications in terms of road network capacity, as is demonstrated by the following section of this report.

Traffic Implications - Road Network Capacity

The traffic implications of planning proposals primarily concern the effects that any *additional* traffic flows may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA NETWORK 8 program which is widely used by the RMS and many LGA's for this purpose. Criteria for evaluating the results of SIDRA analysis are reproduced in the following pages. The Movement Summaries for the traffic analysis are reproduced in Appendix B.

The results of the SIDRA NETWORK analysis of the intersections are summarised in Tables 3.1 and Table 3.2 on the following pages, revealing that:

Beamish Street and Ninth Avenue Intersection (Table 3.1)

- the intersection currently operates at *Levels of Service "A" & "B"* under the existing traffic demands during the commuter peak periods with total average vehicle delays in the order of 14-15 seconds/vehicle
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection is expected to continue to operate at *Level of Service "B"* during the commuter peak periods, with increases in total average vehicle delays of *less than* 1 second/vehicle.

TABLE 3.1 - RESULTS OF SIDRA ANALYSIS OFBEAMISH STREET & NINTH AVENUE					
		Existing Traffic Demand		Projected Development Traffic Demand	
Key Indicators		AM	РМ	AM	PM
Level of Service		В	А	В	А
Degree of Saturation		0.787	0.797	0.839	0.827
Average Vehicle Delay (secs/veh)				
Beamish Street (south)	L T	7.7 17.9	7.6 15.4	77 17.9	7.7 15.4
Beamish Street (north)	T R	7.6 16.3	6.5 14.6	7.6 16.6	6.5 14.9
Ninth Avenue (west)	L R	12.1 26.9	12.8 28.6	12.2 29.5	12.8 30.0
TOTAL AVERAGE VEHICLE	DELAY	14.8	14.0	15.4	14.3
		BEA_	_NINX	BEA	_NINP

Beamish Street and Campsie Street Intersection (Table 3.2)

- the intersection currently operates at Level of Service "A" under the existing traffic demands during the commuter peak periods with total average vehicle delays in the order of 1 second/vehicle. Notwithstanding, the right turn movement out of Campsie Street onto Beamish Street experience delays in the order of 20 seconds/vehicle.
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection is expected to continue to operate at Level of Service "A" during the commuter peak periods, with no change in total average vehicle delays, whilst the right turn movement out of Campsie Street onto Beamish Street increases by just 1 second/vehicle.

Key Indicators		Existing Traffic Demand		Projected Developmen Traffic Demand	
		AM	PM	AM	PM
Level of Service		А	А	А	А
Degree of Saturation		0.324	0.309	0.336	0.314
Average Vehicle Delay (secs/veh)					
Beamish Street (south)	L T	2.9 0.0	2.9 0.0	2.9 0.0	2.9 0.0
Beamish Street (north)	T R	0.0 6.4	0.0 6.1	0.0 6.6	0.0 6.3
Campsie Street (west)	L R	4.7 20.0	4.4 19.1	4.7 21.5	4.4 20.1
TOTAL AVERAGE VEHICLE D	ELAY	1.0	1.3	1.0	1.3

In summary, those projected traffic flows as a consequence of the planning proposal will not have any unacceptable traffic implications in terms of road network capacity, nor will any road upgrades/improvements/widening be required.

Criteria for Interpreting Results of Sidra Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
'E'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
C	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals¹ both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

1

The values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs.

4. PARKING IMPLICATIONS

Existing Kerbside Parking Restrictions

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 6. Key features of those parking restrictions are:

- ¹/₂ HOUR / 1 HOUR PARKING restrictions along selected locations on Beamish Street
- BUS ZONES located at regular intervals along both sides of Beamish Street
- 1 HOUR PARKING restrictions along the Ninth Avenue site frontage
- NO PARKING / LOADING ZONE restrictions along the southern side of Ninth Avenue in the vicinity of the site
- NO STOPPING restrictions along the northern side of Ninth Avenue in the vicinity of the site
- 1 HOUR / 2 HOUR PARKING restrictions along both sides of Campsie Street, including along the entire site frontage.

Off-Street Car Parking Provisions

The off-street car parking requirements applicable to the development proposal are specified in Council's *CDCP 2012*, *Part 6.8 – Parking and Vehicle Access* document in the following terms:

Shop Top Housing (DE House	
Studio apartments:	0.25 spaces per dwelling
1 bedroom apartments:	0.8 spaces per dwelling
2 bedroom apartments:	1 space per dwelling
3 bedroom apartments:	1 space per dwelling
Visitors:	Not required

Shop Top Housing (B2 Zones – Large Centres)



Office Premises (B2 Zones – Large Centres) 1 space per 60m² (120m² – 1,000m²)

Shops (B2 Zones – Large Centres)

1 space per 66.7m² (<120m²) 1 space per 33m² (120m² – 1,000m²) 1 space per 27m² (>1,000m²)

Notwithstanding, the subject site is located within 800 metres of a railway station in the Sydney metropolitan area, and therefore the residential component of the development is also subject to the parking requirements specified in the *State Environmental Planning Policy No* 65 – *Design Quality of Residential Flat Development (Amendment No 3), 2015* in the following terms:

30 Standards that cannot be used to refuse development consent or modification of development consent

- (1) If an application for the modification of a development consent or a development application for the carrying out of development to which this Policy applies satisfies the following design criteria, the consent authority must not refuse the application because of those matters:
 - a) if the car parking for the building will be equal to, or greater than, the recommended minimum amount of car parking specified in Part 3J of the Apartment Design Guide.

Reference is therefore made to the *Apartment Design Guide 2015, Section 3J – Bicycle and Car Parking* document which nominates the following car parking requirements:

Objective 3J-1

Car parking is provided based on proximity to public transport in metropolitan Sydney and centres in regional areas

For development in the following locations:

- on sites that are within 800 metres of a railway station or light rail stop in the Sydney Metropolitan Area; or
- on land zoned, and sites within 400 metres of land zoned, B3 Commercial Core, B4 Mixed Use or equivalent in a nominated regional centre
the minimum car parking requirements for residents and visitors is set out in the Guide to Traffic Generating Developments, or the car parking requirement prescribed by the relevant council, whichever is less.

The car parking needs for a development must be provided off street.

Comparison therefore needs to be drawn between the off-street car parking requirements for residential flat buildings outlined in the *Canterbury DCP 2011* and also the RMS *Guidelines* to determine the *lesser* requirement. The relevant car parking rates outlined in the RMS *Guidelines* are reproduced below:

RMS Guidelines–High Density Residential Flat Buildings in Metropolitan Regional (CBD) Centres

- 0.4 spaces per 1 bedroom unit
- 0.7 spaces per 2 bedroom unit 1.2 spaces per 3 bedroom unit
- 1 space per 7 units for visitor parking

In any event, it is expected that the above numerical car parking requirements will ultimately be satisfied as part of any future development application.

The geometric design layout of the future car parking facilities will ultimately be designed to comply with Standards Australia publication *Parking Facilities Part 1 - Off-Street Car Parking AS2890.1* and *Parking Facilities Part 6 - Off-Street Parking for People with Disabilities AS2890.6*.

Off-Street Bicycle Parking Provisions

The off-street bicycle parking requirements applicable to the development proposal are also specified in the *Canterbury DCP 2012* document in the following terms:

Residential Accommodation

Residents:	1 space per 5 dwellings or part thereof
Visitors:	1 space per 10 dwellings or part thereof

Shop, Restaurant or Cafe

Staff:	1 space per 200m ² or part thereof
Patrons:	1 space per $500m^2$ over $1,000m^2$ or part thereof

The proposed development will ultimately provide the required number of bicycle parking spaces, thereby ensuring the development's commitment to a more sustainable approach to travel.

Loading/Servicing Provisions

The proposed new development is expected to be serviced by a variety of light commercial vehicles and rigid trucks up to and including medium rigid trucks. The loading dock and manoeuvring area will ultimately be designed to accommodate the swept turning path requirements of these trucks, allowing them to enter and exit the site in a forward direction at all times, noting that the loading area will include a mechanical turntable.

The geometric design layout of the proposed loading facilities will ultimately be designed to comply with the relevant requirements specified in the Standards Australia publication *Parking Facilities Part 2 - Off-Street Commercial Vehicle Facilities AS2890.2* in respect of overhead clearances, loading dock dimensions and service area requirements for MRV trucks.

Conclusion

Based on the analysis and discussions presented within this report, the following conclusions are made:

- the planning proposal seeks approval to amend the planning controls of the site to permit the redevelopment of the site to comprise up to 320 apartments, approximately 3,145m² of retail floor space and 1,600m² of commercial floor space
- the capacity analysis of nearby intersections using the SIDRA capacity analysis program indicates that:
 - the projected additional traffic flows will not have any adverse effects on the operational performance of the nearby intersections, and

- no road improvements or intersection upgrades would be required as a consequence of the planning proposal
- the future car parking and loading facilities will be provided and designed in accordance with Council's requirements and the relevant Australian Standards
- the future bicycle parking facilities will be provided and designed in accordance with Council's requirements.

It is therefore reasonable to conclude that the planning proposal will not have any unacceptable implications in terms of road network capacity, access or off-street parking/loading requirements.

APPENDIX A

TRAFFIC SURVEY DATA

	Reliat	ole. Or	iainal	& Auth	nentic l	Result	s	PEDS	NO	RTH	W	EST	SOL	JTH		PEDS	NOF	RTH	WE	ST	SOUT	H	1
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								0900 - 0915	-	7	3	32	()	39	PEAK HR	1	0	18	37	0		19
								0915 - 0930	Ę	5	3	88	()	43								-
								Per End	3	8	4	44	с.,	3	485								
<u>Lights</u>	NO	тн	W	EST	SO	UTH		Heavies	NO	RTH	W	EST	SOL	ітн		Combined	NO	RTH	WE	ST	SOUT	.н	-
Ligitta	Beam			h Ave		ish St		<u>neavies</u>	-	ish St		h Ave	Beam	-		combined	-	ish St	Ninth	-	Beamis		1
Time Per	Т	R	1	R	L	Т	тот	Time Per	 T	R	1	R	1	Т	тот	Time Per	Т	R	1	R	1	т	то
630 - 0645	63	15	46	69	99	77	369	0630 - 0645	5	0	0	1	0	5	11	0630 - 0645	68	15	46	70	99	82	3
645 - 0700	57	7	46	65	87	57	319	0645 - 0700	5	0	0	0	1	5	11	0645 - 0700	62	7	46	65	88	62	33
700 - 0715	58	9	47	80	65	50	309	0700 - 0715	5	0	0	1	0	6	12	0700 - 0715	63	9	47	81	65	56	32
0715 - 0730	47	15	29	62	71	57	281	0715 - 0730	5	0	0	0	2	5	12	0715 - 0730	52	15	29	62	73	62	29
730 - 0745	55	21	61	86	77	65	365	0730 - 0745	6	0	0	0	1	4	11	0730 - 0745	61	21	61	86	78	69	37
0745 - 0800	62	25	59	79	84	71	380	0745 - 0800	6	0	0	0	2	9	17	0745 - 0800	68	25	59	79	86	80	39
0800 - 0815	66	28	55	62	71	59	341	0800 - 0815	6	0	0	0	0	5	11	0800 - 0815	72	28	55	62	71	64	35
0815 - 0830	77	25	77	86	79	89	433	0815 - 0830	3	0	0	0	0	7	10	0815 - 0830	80	25	77	86	79	96	44
0830 - 0845	97	43	83	124	89	75	511	0830 - 0845	5	0	0	0	0	6	11	0830 - 0845	102	43	83	124	89	81	52
0845 - 0900	88	27	30	90	59	71	365	0845 - 0900	4	0	0	2	1	1	8	0845 - 0900	92	27	30	92	60	72	37
0900 - 0915	55	37	42	68	55	75	332	0900 - 0915	7	0	0	0	1	2	10	0900 - 0915	62	37	42	68	56	77	34
0915 - 0930	86	29	45	71	68	55	354	0915 - 0930	2	0	0	1	0	5	8	0915 - 0930	88	29	45	72	68	60	36
Per End	811	281	620	942	904	801	4359	Per End	59	0	0	5	8	60	132	Per End	870	281	620	947	912	861	44
Lights	NO	ктн	w	EST	SO	UTH		Heavies	NO	RTH	W	EST	SOL	JTH		Combined	NOF	RTH	WE	ST	SOUT	Ĥ	1
	Beam		Nint	h Ave	Beam	ish St			Beam	ish St	Nint	h Ave	Beam	ish St			Beam	ish St	Ninth	Ave	Beamis	h St	-
Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	т	тс
630 - 0730	225	46	168	276	322	241	1278	0630 - 0730	20	0	0	2	3	21	46	0630 - 0730	245	46	168	278	325	262	13
645 - 0745	217	52	183	293	300	229	1274	0645 - 0745	21	0	0	1	4	20	46	0645 - 0745	238	52	183	294	304	249	13
700 - 0800	222	70	196	307	297	243	1335	0700 - 0800	22	0	0	1	5	24	52	0700 - 0800	244	70	196	308		267	13
715 - 0815	230	89	204	289	303	252	1367	0715 - 0815	23	0	0	0	5	23	51	0715 - 0815	253	89	204	289	308	275	14
730 - 0830	260	99	252	313	311	284	1519	0730 - 0830	21	0	0	0	3	25	49	0730 - 0830	281	99	252	313	314	309	15
745 - 0845	302	121	274	351	323	294	1665	0745 - 0845	20	0	0	0	2	27	49	0745 - 0845	322	121	274	351	325	321	17
800 - 0900	328	123	245	362	298	294	1650	0800 - 0900	18	0	0	2	1	19	40	0800 - 0900	346	123	245	364	299	313	16
815 - 0915	317	132	232	368	282	310	1641	0815 - 0915	19	0	0	2	2	16	39	0815 - 0915	336	132	232	370	284	326	16
830 - 0930	326	136	200	353	271	276	1562	0830 - 0930	18	0	0	3	2	14	37	0830 - 0930	344	136	200	356	273	290	15
	302	121	274	351	323	294	1665	PEAK HR	20	0	0	0	2	27	49	PEAK HR	322	121	274	351	325	321	17

Reliable, Original & Authen	tia Baaulta						Client Job No/Na		: Varga Traffi : 6946 CAMF		
Ph.88196847, Mob.0418-239	9019		. 1	0			Day/Dat	e	: Thursday 1	St NOVE	ember 2018
			1	2	3						
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			4	5	6						
									AL VOLUMES		
			7	8	9						
<u>AM PEAK</u>									PERIOD		
0745 - 0845					N						
				7							
	Bea	mish St			•					Beam	ish St
											u
	A		20								
		42	23								
	595	44	3							1481	
	568 0	20									59
	27 121	302								1421	
	121	322									1092
Ninth Ave										60	
0 625 625	▲ ◀'	•									1151
0 274 274 -	- P						5 1562		1567		
	Å ()	1,0,*				Ninth A	ve				★
0 351 351 -		↑			▲ 1	193	1185	8			
446 444 2										1773	
		321									64
	325	294								1705	
	323	27									1753
	646 2		20							68	
	617	65	53								1817
	29	67	′3								
											↓
										-	
	Bea	mish St								веат	ish St

9 . A		A.R.			hentic	Resu	ts	PEDS	NO	RTH	W	EST	SO	JTH		PEDS	NO	RTH	WE	ST	SOU	ЛТН	1
			•		-23901			Time Per	-	ish St		h Ave		ish St	тот	Peak Per	-	ish St	Ninth	-	Beami	-	то
			-					1530 - 1545	1	0	6	63		0	73	1530 - 1630	2	2	2.	48	4		274
								1545 - 1600		3	6	69	:	3	75	1545 - 1645	2	21	2	53	4		278
Client	t	: Varg	a Traf	fic Plar	nning			1600 - 1615		5	5	55	(0	60	1600 - 1700	2	29	2	68	3		30
Job No/N	ame	: 6946	CAN	IPSIE E	Beamis	sh St		1615 - 1630		4	6	61		1	66	1615 - 1715	2	29	2	70	3	i	30
Day/Da	ate	: Thur	sdav	1st Nov	vembei	r 2018		1630 - 1645		9	6	68		0	77	1630 - 1730	2	27	3	13	2		34
								1645 - 1700	1	1		34	:	2	97	1645 - 1745	2	9	3	26	2		35
								1700 - 1715		5	5	57		0	62	1700 - 1800	2	28	3	19	0		34
								1715 - 1730		2	1	04	(0	106	1715 - 1815	3	33	34	42	0		37
								1730 - 1745	1	1	8	31		0	92	1730 - 1830	3	6	3:	33	5		37
								1745 - 1800		0		77		0	87								
								1800 - 1815		0		30		0	90	PEAK HR	2	9	3	26	2		35
								1815 - 1830		5		95		5	105								
								Per End	٤	85	8	94	1	1	990								
Lights	NO	ЯΤΗ	w	EST	SO	UTH		Heavies	NO	RTH	W	EST	SO	JTH		Combined	NO	RTH	WE	ST	SOU	ЛТН	1
	Beam	ish St	Nint	h Ave	Beam	ish St			Beam	ish St	Nint	h Ave	Beam	ish St			Beam	ish St	Ninth	Ave	Beami	sh St	1
Time Per	Ι	<u>R</u>	L	<u>R</u>	L	I	тот	Time Per	I	<u>R</u>	L	<u>R</u>	L	T	тот	Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	I	тс
530 - 1545	95	36	43	68	82	49	373	1530 - 1545	5	1	0	0	0	4	10	1530 - 1545	100	37	43	68	82	53	38
545 - 1600	86	45	33	68	59	57	348	1545 - 1600	8	0	0	0	0	5	13	1545 - 1600	94	45	33	68	59	62	30
600 - 1615	68	59	37	75	63	68	370	1600 - 1615	8	0	0	0	0	4	12	1600 - 1615	76	59	37	75	63	72	38
615 - 1630	95	50	35	55	67	71	373	1615 - 1630	8	0	0	0	0	5	13	1615 - 1630	103	50	35	55	67	76	38
630 - 1645	93	42	39	92	88	73	427	1630 - 1645	6	0	0	0	0	4	10	1630 - 1645	99	42	39	92	88	77	43
645 - 1700	103	32	42	75	68	68	388	1645 - 1700	2	0	0	0	0	2	4	1645 - 1700	105	32	42	75	68	70	39
700 - 1715	69	43	44	61	74	77	368	1700 - 1715	7	0	0	0	0	2	9	1700 - 1715	76	43	44	61	74	79	37
715 - 1730	83	59	33	68	67	79	389	1715 - 1730	4	0	0	0	2	7	13	1715 - 1730	87	59	33	68	69	86	40
730 - 1745 745 - 1800	110 68	53 29	47 36	86 61	99 68	34 95	429 357	1730 - 1745 1745 - 1800	2	0	0	0	1 0	6	14 9	1730 - 1745 1745 - 1800	116 70	53 30	47 36	87 61	100 68	40 101	44
800 - 1815	53	44	30	50	61	82	329	1800 - 1815	4	0	0	0	0	4	8	1800 - 1815	57	44	39	50	61	86	33
815 - 1830	95	37	43	61	63	86	385	1815 - 1830	2	0	0	1	2	5	10	1815 - 1830	97	37	43	62	65	91	39
Per End	1018	529	471	820	859	839	4536	Per End	62	2	0	2	5	54	125	Per End	1080	531	471	822	864	893	46
				- OT						DTU	14/7	-07							14/5	OT			
<u>Lights</u>	NO Beam			EST h Ave		UTH hish St		<u>Heavies</u>		RTH ish St		EST h Ave		UTH ish St		<u>Combined</u>		RTH ish St	Ninth	ST	SOU Beami		
Peak Per	T	R	1	R	Deam	T	тот	Peak Per	T	R	1	R	Deam	T	тот	Peak Per	T	R	1	R	Jeann	<i>зл 31</i> Т	тс
530 - 1630	344	190	<u>∟</u> 148	266	271	245	1464	1530 - 1630	<u>1</u> 29	1	0	0	0	18	48	1530 - 1630	<u>1</u> 373	191	148	<u>×</u> 266	<u>⊾</u> 271	<u>1</u> 263	15
545 - 1645	342	196	144	290	277	269	1518	1545 - 1645	30	0	0	0	0	18	48	1545 - 1645	372	196	144	290	277	287	15
600 - 1700	359	183	153	297	286	280	1558	1600 - 1700	24	0	0	0	0	15	39	1600 - 1700	383	183	153	297	286	295	15
615 - 1715	360	167	160	283	297	289	1556	1615 - 1715	23	0	0	0	0	13	36	1615 - 1715	383	167	160	283	297	302	15
630 - 1730	348	176	158	296	297	297	1572	1630 - 1730	19	0	0	0	2	15	36	1630 - 1730	367	176	158	296	299	312	16
645 - 1745	365	187	166	290	308	258	1574	1645 - 1745	19	0	0	1	3	17	40	1645 - 1745	384	187	166	291	311	275	16
700 - 1800	330	184	160	276	308	285	1543	1700 - 1800	19	1	0	1	3	21	45	1700 - 1800	349	185	160	277	311	306	15
715 - 1815	314	185	155	265	295	290	1504	1715 - 1815	16	1	0	1	3	23	44	1715 - 1815	330	186	155	266	298	313	15
730 - 1830	326	163	165	258	291	297	1500	1730 - 1830	14	1	0	2	3	21	41	1730 - 1830	340	164	165	260	294	318	15
		187	166	290	308	258	1574	PEAK HR	19	0	0	1	3	17	40	PEAK HR	384	187	166	291	311	275	16

R		A.R.														Clier			a Traffic		
	Reliab	ole, Orig	ginal &	& Auth	entic F	Result	s								J	ob No/ľ	Name	: 6946	CAMPS	IE Be	amish St
DA	Ph.881	196847	, Mob.	.0418-2	239019	9										Day/D	ate	: Thur	sday 1st	Nove	mber 2018
										1	2		3								
										4	5		6								
																	тот	AL VOLI	JMES		
										7	8		9				F	OR COU	NT		
	<u> PM P</u>	<u>EAK</u>																PERIO)		
	1645 -	1745										N									
							Beam	ish St				v							В	eami	sh St
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						Т			552												
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					1		187	365											1	310	
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	1	456	457 -		^	•	_1	*													1611
		0	166	166				\							2	1291		1293			
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<u> </u>	498	495 3	3			•													1	757	
								275													64
							311	258											1	698	1000
						•	308	17													1838
							3		20											59	1000
						566			655												1902
					2	20			675												\blacksquare
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							Beam	ish St											В	eami	sh St



9/ 20 14		ble, Or	DA		ontio	Dooult	•	PEDS	NO	RTH	14/1	EST	SOL	пти		PEDS	NO	этц	WE	ет	SOU	ти	1
		196847	-				5	Time Per	-	ish St		col osie St	Beam		тот	Peak Per	-	ish St	Camp	-	Beamis		тот
DA	F 11.00	190041	, 1000	.0410-	239013	9		0630 - 0645		0		35)	35	0630 - 0730)	14 14		0		147
								0645 - 0700		0		25)	25	0645 - 0745)	15		0		147
Clien	t.	· Vara	a Traff	ic Plan	nina			0700 - 0715		0		40))	40	0700 - 0800			20		1		206
					leamis	- Ct										-							
Job No/N			-	-				0715 - 0730		0		47)	47	0715 - 0815			24	-	1		250
Day/Da	ate	: Thur	sday 1	st Nov	ember	2018		0730 - 0745		0		45)	45	0730 - 0830		3	26		1		269
								0745 - 0800		1		72	1	•	74	0745 - 0845		3	28		1		290
								0800 - 0815		0		34)	84	0800 - 0900		3	29		1		294
								0815 - 0830		2		64)	66	0815 - 0915		1	28		2		295
								0830 - 0845		0 1		66 76	1)	66 78	0830 - 0930	2	1	29	94	2		300
								0845 - 0900		1		33	1		78 85	PEAK HR		3	29	0	1		294
								0900 - 0915						•		FEAN IIK)	2:	JU	1		294
								0915 - 0930 Per End		2 7		69 1 06) 3	71 716								
															710								
Lights	NO	RTH	W	EST	SO	UTH		Heavies	NO	RTH	W	EST	SOL	JTH		Combined	NO	RTH	WE	ST	SOU	ΤН	
	Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	sie St	Beamis	h St	
Time Per	I	<u>R</u>	L	<u>R</u>	L	I	тот	Time Per	I	R	L	<u>R</u>	L	I	тот	Time Per	Ι	R	L	<u>R</u>	L	Ι	тот
0630 - 0645	118	14	11	9	6	166	324	0630 - 0645	5	1	0	0	1	5	12	0630 - 0645	123	15	11	9	7	171	336
0645 - 0700	113	9	17	7	3	125	274	0645 - 0700	4	1	1	0	0	5	11	0645 - 0700	117	10	18	7	3	130	285
0700 - 0715	130	7	11	3	2	109	262	0700 - 0715	5	1	0	0	1	6	13	0700 - 0715	135	8	11	3	3	115	275
0715 - 0730	99	12	8	12	5	118	254	0715 - 0730	5	0	1	0	0	6	12	0715 - 0730	104	12	9	12	5	124	266
0730 - 0745	135	6	14	9	11	129	304	0730 - 0745	5	1	0	0	0	5	11	0730 - 0745	140	7	14	9	11	134	315
0745 - 0800	130	10	8	5	17	148	318	0745 - 0800	6	0	0	0	1	10	17	0745 - 0800	136	10	8	5	18	158	335
0800 - 0815	115	12	16	9	13	115	280	0800 - 0815	3	3	0	0	0	5	11	0800 - 0815	118	15	16	9	13	120	291
0815 - 0830	146	15	35	9	8	136	349	0815 - 0830	3	0	0	0	0	7	10	0815 - 0830	149	15	35	9	8	143	359
0830 - 0845	210	11	19	5	11	146	402	0830 - 0845	4	1	0	0	2	6	13	0830 - 0845	214	12	19	5	13	152	415
0845 - 0900	155	23	17	11	13	115	334	0845 - 0900	6	0	0	0	2	5	13	0845 - 0900	161	23	17	11	15	120	347
0900 - 0915	115	8	14	5	11	118	271	0900 - 0915	6	1	0	0	1	3	11	0900 - 0915	121	9	14	5	12	121	282
0915 - 0930	135	20	12	6	15	113	301	0915 - 0930	3	0	0	0	2	5	10	0915 - 0930	138	20	12	6	17	118	311
Per End	1601	147	182	90	115	1538	3673	Per End	55	9	2	0	10	68	144	Per End	1656	156	184	90	125	1606	3817
Lights	NO	RTH	w	EST	SO	UTH		Heavies	NO	RTH	w	EST	SOL	ЈТН		Combined	NO	ктн	WE	ST	SOU	тн	
	Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	sie St	Beamis	h St	
Peak Per	Т	R	L	R	L	Т	тот	Peak Per	I	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	Τ	тот
0630 - 0730	460	42	47	31	16	518	1114	0630 - 0730	19	3	2	0	2	22	48	0630 - 0730	479	45	49	31	18	540	1162
0645 - 0745	477	34	50	31	21	481	1094	0645 - 0745	19	3	2	0	1	22	47	0645 - 0745	496	37	52	31	22	503	1141
0700 - 0800	494	35	41	29	35	504	1138	0700 - 0800	21	2	1	0	2	27	53	0700 - 0800	515	37	42	29	37	531	1191
0715 - 0815	479	40	46	35	46	510	1156	0715 - 0815	19	4	1	0	1	26	51	0715 - 0815	498	44	47	35	47	536	1207
0730 - 0830	526	43	73	32	49	528	1251	0730 - 0830	17	4	0	0	1	27	49	0730 - 0830	543	47	73	32	50	555	1300
0745 - 0845	601	48	78	28	49	545	1349	0745 - 0845	16	4	0	0	3	28	51	0745 - 0845	617	52	78	28	52	573	1400
0800 - 0900	626	61	87	34	45	512	1365	0800 - 0900	16	4	0	0	4	23	47	0800 - 0900	642	65	87	34	49	535	1412
0815 - 0915	626	57	85	30	43	515	1356	0815 - 0915	19	2	0	0	5	21	47	0815 - 0915	645	59	85	30	48	536	1403
0830 - 0930	615	62	62	27	50	492	1308	0830 - 0930	19	2	0	0	7	19	47	0830 - 0930	634	64	62	27	57	511	1355
PEAK HR	626	61	87	34	45	512	1365	PEAK HR	16	4	0	0	4	23	47	PEAK HR	642	65	87	34	49	535	1412

			DAT		ntic P	!*										Clie		: Varga Traf		
The F	Reliabl	e, Ori	ginal &	Auther	ntic R	esult	S									Job No		: 6946 CAM		
	'n.881	96847	, Mob.C	0418-23	9019						-		_			Day/I	Date	: Thursday 1	st Nov	ember 2018
										1	2		3							
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										4	5	_	6						-	
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	AM P																	PERIOD	_	
0	- 0080	0900										N								
												75							_	
							Beam	ish St											Beam	ish St
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									68										1700	
						622			70	7									1790	
						599	4	16	•										4700	64
					2	23	61	626											1720	
0		<u></u>					65 I	642											70	1748
Car	mpsie		404	-		•		•											70	1812
	0	121	121 -	-		•									2	27	2	274		1812
		0	87	87 -										Com	∠ npsie St		2	2/4		
)e)						Calli	ipsie Si					•
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Oliant			- T 4					1545 - 1600		3		14)	117	1545 - 1645		6		29	1		436
Client			a Traf		•			1600 - 1615		1	ç	93)	94	1600 - 1700		3	43	37	8		448
Job No/N	ame		6 CAM	-				1615 - 1630		2	1	15	()	117	1615 - 1715		7	46	51	9		477
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								1645 - 1700		0	1	22	7	7	129	1645 - 1745	(6	5	15	8		529
								1700 - 1715		5	1	17		1	123	1700 - 1800	(6	5	56	2		564
								1715 - 1730		1	1	39	(140	1715 - 1815		2	58	31	1		584
								1730 - 1745		0		37)	137	1730 - 1830		6	58	32	3		591
								1745 - 1800		0		63		•	164								
								1800 - 1815		1		42)	143	PEAK HR	(6	48	35	9		500
								1815 - 1830		5		40		2	147								
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Time Per	т	R	L	R	L	Т	тот	Time Per	т	R	L	R	L	Т	тот	Time Per	Т	R	L	R	L	т	тот
1530 - 1545	133	28	14	9	20	114	318	1530 - 1545	5	1	0	0	2	4	12	1530 - 1545	138	29	14	9	22	118	330
1545 - 1600	126	23	15	12	17	102	295	1545 - 1600	6	2	0	0	0	5	13	1545 - 1600	132	25	15	12	17	107	308
1600 - 1615	130	14	18	10	23	116	311	1600 - 1615	8	0	0	0	0	4	12	1600 - 1615	138	14	18	10	23	120	323
1615 - 1630	124	24	27	3	24	113	315	1615 - 1630	7	1	0	0	0	5	13	1615 - 1630	131	25	27	3	24	118	328
1630 - 1645	161	22	28	9	21	135	376	1630 - 1645	5	1	0	0	1	4	11	1630 - 1645	166	23	28	9	22	139	387
1645 - 1700	149	24	27	4	19	114	337	1645 - 1700	1	1	0	0	0	2	4	1645 - 1700	150	25	27	4	19	116	341
1700 - 1715	118	15	17	10	11	133	304	1700 - 1715	5	1	0	0	1	2	9	1700 - 1715	123	16	17	10	12	135	313
1715 - 1730	144	10	35	9	20	110	328	1715 - 1730	5	0	0	0	1	9	15	1715 - 1730	149	10	35	9	21	119	343
1730 - 1745	175	26	25	8	14	108	356	1730 - 1745	4	2	0	0	1	7	14	1730 - 1745	179	28	25	8	15	115	370
1745 - 1800	114	12	27	3	13	137	306	1745 - 1800	4	0	0	0	0	6	10	1745 - 1800	118	12	27	3	13	143	316
1800 - 1815	100	18	22	7	11	123	281	1800 - 1815	3	1	0	0	2	4	10	1800 - 1815	103	19	22	7	13	127	291
1815 - 1830	130	24	30	13	19	120	336	1815 - 1830	3	0	0	0	1	7	11	1815 - 1830	133	24	30	13	20	127	347
Per End	1604	240	285	97	212	1425	3863	Per End	56	10	0	0	9	59	134	Per End	1660	250	285	97	221	1484	3997
Lights	NO	RTH	WE	ST	so	UTH		Heavies	NO	RTH	w	EST	SOL	ЛТН		Combined	NO	RTH	WE	ST	SOU	тн	
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Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	т	тот	Peak Per	Т	R	L	R	L	т	тот
1530 - 1630	<u>.</u> 513	89	74	34	84	445	1239	1530 - 1630	26	4	0	0	2	18	50	1530 - 1630	539	93	74	34	86	<u>.</u> 463	1289
1545 - 1645	541	83	88	34	85	466	1297	1545 - 1645	26	4	0	0	1	18	49	1545 - 1645	567	87	88	34	86	484	1346
1600 - 1700	564	84	100	26	87	478	1339	1600 - 1700	21	3	0	0	1	15	40	1600 - 1700	585	87	100	26	88	493	1379
1615 - 1715		85	99	26	75	495	1332	1615 - 1715	18	4	0	0	2	13	37	1615 - 1715		89	99	26	77	508	1369
1630 - 1730	572	71	107	32	71	492	1345	1630 - 1730	16	3	0	0	3	17	39	1630 - 1730		74	107	32	74	509	1384
1645 - 1745	586	75	104	31	64	465	1325	1645 - 1745	15	4	0	0	3	20	42	1645 - 1745		79	104	31	67	485	1367
1700 - 1800	551	63	104	30	58	488	1294	1700 - 1800	18	3	0	0	3	24	48	1700 - 1800	569	66	104	30	61	512	1342
1715 - 1815	533	66	109	27	58	478	1271	1715 - 1815	16	3	0	0	4	26	49	1715 - 1815		69	109	27	62	504	1320
1730 - 1830	519	80	104	31	57	488	1279	1730 - 1830	14	3	0	0	4	24	45	1730 - 1830	533	83	104	31	61	512	1324
	572	71	107	32	71	492	1345	PEAK HR	16	3	0	0	3	17	39	PEAK HR	588	74	107	32	74	509	1384

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						Bean	nish St											Beam	ish St
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APPENDIX B

SIDRA MOVEMENT SUMMARIES

Site: 101 [BEA_NINX AM]

♦♦ Network: N101 [Existing Network AM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	ement	Performa	ince ·	- Vehio	cles									
Mov ID	Turn	Demand F				Deg. Satn	Average Delay	Level of Service	Aver. Back Queue		Prop. Queued	Effective Stop	Aver. A No.	e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dista veh	ance m		Rate	Cycles S	Speed km/h
South	i: Bean	nish St (S)												
1	L2	325	0.6	325	0.6	0.275	7.7	LOS A	2.1	15.0	0.47	0.64	0.47	30.2
2	T1	321	8.4	321	8.4	0.620	17.9	LOS B	4.4	33.1	0.92	0.79	0.95	21.1
Appro	bach	646	4.5	646	4.5	0.620	12.7	LOS A	4.4	33.1	0.69	0.72	0.70	25.1
North	: Bearr	nish St (N)												
8	T1	322	6.2	322	6.2	0.330	7.6	LOS A	2.8	20.9	0.62	0.53	0.62	26.8
9	R2	121	0.0	121	0.0	0.279	16.3	LOS B	1.3	9.4	0.84	0.74	0.84	26.4
Appro	bach	443	4.5	443	4.5	0.330	10.0	LOS A	2.8	20.9	0.68	0.58	0.68	26.6
West	Ninth	Ave (W)												
10	L2	274	0.0	274	0.0	0.307	12.1	LOS A	2.5	17.8	0.65	0.71	0.65	29.3
12	R2	351	0.0	351	0.0	0.787	26.9	LOS B	5.7	40.0	0.99	0.98	1.24	16.1
Appro	bach	625	0.0	625	0.0	0.787	20.4	LOS B	5.7	40.0	0.84	0.86	0.98	21.8
All Ve	hicles	1714	2.9	1714	2.9	0.787	14.8	LOS B	5.7	40.0	0.74	0.74	0.80	24.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P3	North Full Crossing	10	19.4	LOS B	0.0	0.0	0.88	0.88
P4	West Full Crossing	187	19.5	LOS B	0.2	0.2	0.89	0.89
All Pe	destrians	197	19.5	LOS B			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [BEA_NINX PM]

♦♦ Network: N101 [Existing Network PM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	ement	Performa	ince ·	- Vehi	cles									
Mov ID	Turn	Demand F				Deg. Satn	Average Delay	Level of Service	Aver. Bao Queu		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Di veh	stance m		Rate	Cycles S	Speed km/h
South	n: Bean	nish St (S)												
1	L2	299	0.7	299	0.7	0.253	7.6	LOS A	1.9	13.6	0.46	0.64	0.46	30.3
2	T1	312	4.8	312	4.8	0.516	15.4	LOS B	3.9	28.7	0.86	0.73	0.86	22.5
Appro	bach	611	2.8	611	2.8	0.516	11.6	LOS A	3.9	28.7	0.66	0.68	0.66	26.0
North	: Beam	nish St (N)												
8	T1	367	5.2	367	5.2	0.347	6.5	LOS A	3.0	22.2	0.58	0.50	0.58	28.1
9	R2	176	0.0	176	0.0	0.365	14.6	LOS B	1.9	13.0	0.82	0.75	0.82	27.4
Appro	bach	543	3.5	543	3.5	0.365	9.1	LOS A	3.0	22.2	0.66	0.58	0.66	27.7
West	Ninth	Ave (W)												
10	L2	158	0.0	158	0.0	0.193	12.8	LOS A	1.5	10.3	0.65	0.69	0.65	28.8
12	R2	296	0.0	296	0.0	0.797	28.6	LOS C	4.9	34.5	1.00	1.00	1.30	15.5
Appro	bach	454	0.0	454	0.0	0.797	23.1	LOS B	4.9	34.5	0.88	0.89	1.07	20.0
All Ve	hicles	1608	2.2	1608	2.2	0.797	14.0	LOS A	4.9	34.5	0.72	0.71	0.78	24.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P3	North Full Crossing	27	19.4	LOS B	0.0	0.0	0.88	0.88
P4	West Full Crossing	313	19.6	LOS B	0.4	0.4	0.89	0.89
All Pe	destrians	340	19.6	LOS B			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [BEA_NINP AM]

♦♦ Network: N101 [Proposed Network AM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	ement	Performa	ince ·	- Vehio	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Back Queue	of	Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dist veh	ance m		Rate	Cycles S	Speed km/h
South	n: Bean	nish St (S)												
1	L2	354	0.6	354	0.6	0.299	7.7	LOS A	2.4	16.7	0.47	0.65	0.47	30.1
2	T1	321	8.4	321	8.4	0.620	17.9	LOS B	4.4	33.1	0.92	0.79	0.95	21.1
Appro	bach	675	4.3	675	4.3	0.620	12.6	LOS A	4.4	33.1	0.69	0.72	0.70	25.3
North	: Beam	nish St (N)												
8	T1	322	6.2	322	6.2	0.330	7.6	LOS A	2.8	20.9	0.62	0.53	0.62	26.8
9	R2	146	0.0	146	0.0	0.338	16.6	LOS B	1.7	11.6	0.86	0.75	0.86	26.3
Appro	bach	468	4.3	468	4.3	0.338	10.4	LOS A	2.8	20.9	0.69	0.60	0.69	26.5
West	Ninth	Ave (W)												
10	L2	296	0.0	296	0.0	0.332	12.2	LOS A	2.8	19.5	0.66	0.71	0.66	29.2
12	R2	374	0.0	374	0.0	0.839	29.5	LOS C	6.5	45.6	1.00	1.06	1.37	15.2
Appro	bach	670	0.0	670	0.0	0.839	21.9	LOS B	6.5	45.6	0.85	0.90	1.05	21.1
All Ve	hicles	1813	2.7	1813	2.7	0.839	15.4	LOS B	6.5	45.6	0.75	0.76	0.83	23.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P3	North Full Crossing	10	19.4	LOS B	0.0	0.0	0.88	0.88
P4	West Full Crossing	187	19.5	LOS B	0.2	0.2	0.89	0.89
All Pe	destrians	197	19.5	LOS B			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [BEA_NINP PM]

♦♦ Network: N101 [Proposed Network PM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	ement	Performa	ince ·	- Vehio	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Back Queue	c of	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	tance m		Rate	Cycles S	Speed km/h
South	n: Bear	nish St (S)												
1	L2	324	0.6	324	0.6	0.274	7.7	LOS A	2.1	15.0	0.47	0.64	0.47	30.2
2	T1	312	4.8	312	4.8	0.516	15.4	LOS B	3.9	28.7	0.86	0.73	0.86	22.5
Appro	bach	636	2.7	636	2.7	0.516	11.5	LOS A	3.9	28.7	0.66	0.68	0.66	26.1
North	: Bean	nish St (N)												
8	T1	367	5.2	367	5.2	0.347	6.5	LOS A	3.0	22.2	0.58	0.50	0.58	28.1
9	R2	201	0.0	201	0.0	0.419	14.9	LOS B	2.2	15.3	0.84	0.77	0.84	27.2
Appro	bach	568	3.3	568	3.3	0.419	9.5	LOS A	3.0	22.2	0.67	0.60	0.67	27.6
West	: Ninth	Ave (W)												
10	L2	169	0.0	169	0.0	0.207	12.8	LOS A	1.6	11.1	0.65	0.69	0.65	28.8
12	R2	307	0.0	307	0.0	0.827	30.0	LOS C	5.3	37.0	1.00	1.05	1.37	15.0
Appro	bach	476	0.0	476	0.0	0.827	23.9	LOS B	5.3	37.0	0.88	0.92	1.12	19.7
All Ve	hicles	1680	2.1	1680	2.1	0.827	14.3	LOS A	5.3	37.0	0.73	0.72	0.79	24.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P3	North Full Crossing	27	19.4	LOS B	0.0	0.0	0.88	0.88
P4	West Full Crossing	313	19.6	LOS B	0.4	0.4	0.89	0.89
All Pe	destrians	340	19.6	LOS B			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: 101 [BEA_CAMX AM]

♦♦ Network: N101 [Existing Network AM]

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	nce ·	- Vehi	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Bac Queue		Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	stance m		Rate	Cycles S	Speed km/h
South	n: Bean	nish St (S)			70									
1	L2	52	5.8	52	5.8	0.177	2.9	LOS A	0.0	0.0	0.00	0.07	0.00	39.9
2	T1	573	4.9	573	4.9	0.177	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	37.9
Appro	bach	625	5.0	625	5.0	0.177	0.2	NA	0.0	0.0	0.00	0.04	0.00	38.9
North	: Beam	nish St (N)												
8	T1	617	2.6	617	2.6	0.324	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	52	7.7	52	7.7	0.066	6.4	LOS A	0.1	0.7	0.45	0.62	0.45	35.5
Appro	bach	669	3.0	669	3.0	0.324	0.5	NA	0.1	0.7	0.04	0.05	0.04	38.3
West:	Camp	osie St (W)												
10	L2	78	0.0	78	0.0	0.194	4.7	LOS A	0.3	2.1	0.48	0.61	0.48	33.7
12	R2	28	0.0	28	0.0	0.194	20.0	LOS B	0.3	2.1	0.48	0.61	0.48	33.3
Appro	bach	106	0.0	106	0.0	0.194	8.7	LOS A	0.3	2.1	0.48	0.61	0.48	33.6
All Ve	hicles	1400	3.6	1400	3.6	0.324	1.0	NA	0.3	2.1	0.05	0.09	0.05	37.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [BEA_CAMX PM]

♦♦ Network: N101 [Existing Network PM]

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ince ·	- Vehi	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Bac Queue		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	stance m		Rate	Cycles S	Speed km/h
South	n: Bean	nish St (S)												
1	L2	74	4.1	74	4.1	0.156	2.9	LOS A	0.0	0.0	0.00	0.12	0.00	39.6
2	T1	509	5.5	509	5.5	0.156	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	37.1
Appro	bach	583	5.3	583	5.3	0.156	0.4	NA	0.0	0.0	0.00	0.06	0.00	38.7
North	: Beam	nish St (N)												
8	T1	588	2.7	588	2.7	0.309	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	74	5.4	74	5.4	0.089	6.1	LOS A	0.1	0.9	0.43	0.61	0.43	35.7
Appro	bach	662	3.0	662	3.0	0.309	0.7	NA	0.1	0.9	0.05	0.07	0.05	38.0
West	: Camp	sie St (W)												
10	L2	107	0.0	107	0.0	0.222	4.4	LOS A	0.4	2.6	0.41	0.56	0.41	34.3
12	R2	32	0.0	32	0.0	0.222	19.1	LOS B	0.4	2.6	0.41	0.56	0.41	33.9
Appro	bach	139	0.0	139	0.0	0.222	7.8	LOS A	0.4	2.6	0.41	0.56	0.41	34.2
All Ve	hicles	1384	3.7	1384	3.7	0.309	1.3	NA	0.4	2.6	0.06	0.11	0.06	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [BEA_CAMP AM]

♦ Network: N101 [Proposed **Network AM]**

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ince ·	- Vehi	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Bac Queue		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	stance m		Rate	Cycles S	Speed km/h
South	n: Bean	nish St (S)												
1	L2	52	5.8	52	5.8	0.184	2.9	LOS A	0.0	0.0	0.00	0.07	0.00	39.9
2	T1	599	4.7	599	4.7	0.184	0.0	LOS A	0.0	0.0	0.00	0.03	0.00	38.0
Appro	bach	651	4.8	651	4.8	0.184	0.2	NA	0.0	0.0	0.00	0.04	0.00	38.9
North	: Beam	nish St (N)												
8	T1	640	2.5	640	2.5	0.336	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	52	7.7	52	7.7	0.068	6.6	LOS A	0.1	0.7	0.47	0.63	0.47	35.4
Appro	bach	692	2.9	692	2.9	0.336	0.5	NA	0.1	0.7	0.04	0.05	0.04	38.3
West	: Camp	sie St (W)												
10	L2	78	0.0	78	0.0	0.204	4.7	LOS A	0.3	2.2	0.50	0.62	0.50	33.4
12	R2	28	0.0	28	0.0	0.204	21.5	LOS B	0.3	2.2	0.50	0.62	0.50	33.1
Appro	bach	106	0.0	106	0.0	0.204	9.2	LOS A	0.3	2.2	0.50	0.62	0.50	33.3
All Ve	hicles	1449	3.5	1449	3.5	0.336	1.0	NA	0.3	2.2	0.05	0.08	0.05	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [BEA_CAMP PM]

♦♦ Network: N101 [Proposed Network PM]

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ince ·	- Vehi	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Bac Queue		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	stance m		Rate	Cycles S	Speed km/h
South	n: Bean	nish St (S)												
1	L2	74	4.1	74	4.1	0.162	2.9	LOS A	0.0	0.0	0.00	0.11	0.00	39.6
2	T1	534	5.2	534	5.2	0.162	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	37.2
Appro	bach	608	5.1	608	5.1	0.162	0.4	NA	0.0	0.0	0.00	0.06	0.00	38.7
North	: Beam	nish St (N)												
8	T1	599	2.7	599	2.7	0.314	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	74	5.4	74	5.4	0.091	6.3	LOS A	0.1	0.9	0.44	0.63	0.44	35.6
Appro	bach	673	3.0	673	3.0	0.314	0.7	NA	0.1	0.9	0.05	0.07	0.05	38.0
West	: Camp	sie St (W)												
10	L2	107	0.0	107	0.0	0.230	4.4	LOS A	0.4	2.6	0.43	0.57	0.43	34.1
12	R2	32	0.0	32	0.0	0.230	20.1	LOS B	0.4	2.6	0.43	0.57	0.43	33.7
Appro	bach	139	0.0	139	0.0	0.230	8.0	LOS A	0.4	2.6	0.43	0.57	0.43	34.0
All Ve	hicles	1420	3.6	1420	3.6	0.314	1.3	NA	0.4	2.6	0.07	0.11	0.07	36.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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